

Section 02830

MODULAR CONCRETE RETAINING WALL

PART 1: GENERAL

1.01 Description

- A. Work shall consist of furnishing and constructing a CANDURA[®] Retaining Wall System in accordance with these specifications and in reasonably close conformity with the lines, grades, design, and dimensions shown on the plans.
- B. Work includes preparing foundation soil, furnishing and installing leveling pad, if required, plantable soil unit fill and backfill to the lines and grades shown on the construction drawings.
- A. Work includes furnishing and installing geosynthetic soil reinforcement of the type, size, location, and lengths designated on the construction drawings.
- B. Work includes furnishing and installing foundation drain, sub-drain, and other wall related drainage systems that may be shown on the construction drawings.

1.02 Related Sections

- A. Section 02200 - Site Preparation
- B. Section 02300 - Earthwork

1.03 Reference Documents

- A. American Society for Testing and Materials (ASTM)
 - 1. ASTM C-1372 Specification for Segmental Retaining Wall Units
 - 2. ASTM D-422 Particle Size Analysis
 - 3. ASTM D-698 Laboratory Compaction Characteristics of Soil -Standard Proctor
 - 4. ASTM D-1557 Laboratory Compaction Characteristics of Soil –Modified Proctor
 - 5. ASTM D-4318 Liquid Limits, Plastic Limit and Plasticity Index of Soils
 - 6. ASTM D-4595 Tensile Properties of Geotextiles - Wide Width Strip
 - 7. ASTM D-5262 Unconfined Tension Creep Behavior of Geosynthetics
 - 8. ASTM D-3034 Polyvinyl Chloride Pipe (PVC)
 - 9. ASTM D-1248 Corrugated Plastic Pipe
 - 10. ASTM C 140 Std. Spec. for Sampling and Testing Concrete Masonry Units.
 - 11. ASTM C 145 Std. Spec. for Solid Load Bearing Concrete Masonry Units.
- B. Geosynthetic Research Institute (GRI)
 - 1. GRI-GG4 Determination of Long Tern Design Strength of Geogrids
 - 2. GRI-GT7 Determination of Long Tern Design Strength of Geotextiles
 - 3. GRI-GG5 Determination of Geogrid (soil) Pullout
- C. National Concrete Masonry Association (NCMA)
 - 1. NCMA SRWU-1 Test Method for Determining Connection Strength of SRW
 - 2. NCMA SRWU-2 Test Method for Determining Shear Strength of SRW

1.04 Submittals/Certification

- A. Contractor shall submit a Manufacturer's certification, prior to start of work, that the retaining wall system components meet the requirements of this specification and the structure design.
- B. Contractor shall submit construction drawings and design calculations for the retaining wall system prepared and stamped by a Professional Engineer registered in the state of the project. The engineering designs, techniques, and material evaluations shall be in accordance with the NCMA Design Guidelines For Segmental Retaining Walls, or the AASHTO Standard Specifications for Highway Bridges, Section 5.8 (whichever is applicable to designer).
- C. Contractor shall submit a test report documenting strength of specific modular concrete unit and geosynthetic reinforcement connection to CANDURA[®] blocks. The maximum design tensile load of the geosynthetic shall be equal to the laboratory tested ultimate strength of geosynthetic-facing unit connection at a maximum normal force as is appropriate for the vertical location of reinforcement under consideration. The connection strength evaluation shall be performed in accordance with NCMA test method SRWU-1.

1.05 Quality Assurance

- A. Contractor shall submit certification, prior to start of work, that the contractor has installed modular concrete units and geosynthetic reinforcements on:
 - 1) a minimum of five (5) similar projects, i.e., height, soil fill types, erection tolerances, etc.; and
 - 2) a minimum of 1 million square feet (93,000 m²) of retaining walls.
- B. Contractor shall submit a list of five (5) previously constructed projects of similar size and magnitude by the wall installer where segmental retaining wall systems have been constructed successfully. Contact names and telephone numbers shall be listed for each project.
- C. Contractor shall provide evidence that the design engineer has a minimum of five years of documentable experience in the design for reinforced soil structures. The design engineer shall provide proof of current professional liability insurance with an aggregate coverage limit of not less than \$2,000,000.
- D. Owner shall provide soil testing and quality assurance inspection during earthwork and wall construction operations. Owner's quality assurance program does not relieve the contractor of responsibility for wall performance.

1.06 Delivery, Storage and Handling

- A. Contractor shall check all materials upon delivery to assure that the proper type, grade, color, and certification have been received.
- B. Contractor shall protect all materials from damage due to jobsite conditions and in accordance with manufacturer's recommendations. Damaged materials shall not be incorporated into the work.

PART 2: PRODUCTS

2.01 Definitions

- A. Modular Unit - a concrete retaining wall element machine made from portland cement, water, and aggregates.
- B. Geosynthetic (geogrid and/or geotextile) - a structural element formed by a regular network of woven or woven and coated tensile elements which, when embedded within the soil mass and connected to the modular concrete units, will develop friction and interlock with the surrounding soil, rock, or earth and function primarily as reinforcement.
- C. Unit Plantable Fill – plantable soil which is placed within the modular concrete units.
- D. Reinforced Backfill - compacted soil which is placed within the reinforced soil volume as outlined on the plans.
- E. Geosynthetic/Connector Pipe – Polymeric pipe (typically 1 inch (25 mm) diameter schedule 80 PVC) which is used to interlock and form a positive connection between the block and geosynthetic.
- F. Form Capacity Stakes – High Strength Plastic Stakes that get driven through the block to the underlying soils for form capacity.

2.02 Modular Concrete Retaining Wall Units

- A. Modular concrete units shall conform to the following architectural requirements:

face color/texture – buff concrete with exposed aggregate finish - standard manufacturers' colors may be specified by the Owner.

face finish - Standard, concrete face, with angled profile permitting concave/convex curve installation. Face finish consists of exposed aggregate surface. Other face finishes will not be allowed without written approval of Owner.

bond configuration - running with bonds nominally located at midpoint vertically adjacent units, in both straight and curved alignments.

exposed surfaces of units shall be free of chips, cracks or other imperfections when viewed from a distance of 10 feet (3 m) under diffused lighting.

- B. Modular concrete materials shall conform to the requirements of ASTM C1372 - Standard Specifications for Segmental Retaining Wall Units.

- C. Modular concrete units shall conform to the following structural and geometric requirements measured in accordance with Section 1.03 and other appropriate references:

compressive strength = 4000 psi (27,600KPa) minimum at 28 days;

moisture absorption = 6% maximum for standard weight aggregates;

dimensional tolerances = ± 1/8 inch (3 mm) from nominal unit dimensions (not including exposed aggregate face texture), ±1/8 inch (3 mm) unit height - top and bottom planes;

Unit Type	C35	Corner	Cap
Unit Size, Height, in (mm)	8 (200)	8 (200)	2.5 (65)

Unit Size, Width, in (mm)	18 (450)	18 (450)	18 (450)
Unit Size, Depth, in (mm)	12 (300)	9 (225)	16 (400)
Weight, (typical), lbs (Kg)	80 (35)	50 (23)	30 (14)

geosynthetic/unit peak connection strength - 3400 plf (49.7 KN/m) minimum at 2 psi (13.8 KPa) normal force for geotextiles and 1000 plf (14.6 KN/m) minimum at 2 psi (13.8 KPa) normal force for geogrid.

- D. Modular concrete units shall conform to the following constructability requirements:

Vertical setback = Vertical setback to be adjusted to meet requirements as set forth in construction documents. Vertical setback for CANDURA® products is 2.5 degrees minimum with setbacks sufficient to obtain a batter of up to 35 degrees from the vertical. Where limited planting is desired, plants can be established within blocks that are set back or pulled apart a small distance.

2.03 Geosynthetic-Concrete Block Unit Connectors

- A. Connectors shall be 1 inch (2.5 cm) diameter or greater schedule 80 pipe or equivalent and must be capable of providing positive mechanical interlock between geosynthetic soil reinforcement material (geotextile or geogrid) and block.
- B. Connectors shall be capable of holding the geosynthetic soil reinforcement in the proper design position during geosynthetic pre-tensioning and backfilling procedures.

2.04 Base Leveling Pad Material (if required)

- A. Material shall consist of a compacted crushed stone base or material as shown on the construction drawings.

2.05 Unit Fill

- A. Unit fill shall consist of same soil as used in reinforced soil zone of wall and (if plantings are to be established within units) have sufficient nutrients to sustain plant growth. Unit fill shall conform to the mix characteristics as specified by the landscape architect.

2.06 Reinforced Backfill

- A. Reinforced backfill shall be free of debris and meet the following gradation tested in accordance with ASTM D-422:

<u>Sieve Size</u>	<u>Percent Passing</u>
2 inch (50 mm)	100-75
3/4 inch (20 mm)	100-75
No. 40	0-60
No. 200	0-35

Plasticity Index (PI) <20 and Liquid Limit <40 per ASTM D-4318.

- B. The maximum aggregate size within 6 inches of geosynthetic reinforcement shall be limited to 3/4 inch (20 mm) unless field tests have been performed to evaluate potential strength reductions to the geosynthetic design due to damage during construction.

- C. Material can be site-excavated soils where the above requirements can be met. Unsuitable soils for backfill (high plastic clays or organic soils) shall not be used in the backfill, in the reinforced soil mass, or in the foundation soils.
- D. Contractor shall submit reinforced fill sample and laboratory test results to the Architect/Engineer for approval prior to the use of any proposed reinforced fill material.

E.

2.07 Geogrid Soil Reinforcement

- A. Geosynthetic reinforcement shall consist of geotextiles and/or geogrids manufactured specifically for soil reinforcement applications and shall be manufactured from high tenacity polyester yarn, polypropylene or high density polyethylene. Polyester geogrid shall be knitted from high tenacity polyester filament yarn with a molecular weight exceeding 25,000 Meg/m and a carboxyl end group values less than 30. Polyester geogrid shall be coated with a material which is resistant to peeling, cracking, and stripping.
- B. T_a , Long Term Allowable Tensile Design Load, of the geogrid material shall be determined as follows:

$T_a = T_{ult} / (RF_{cr} * RF_d * RF_{id} * FS)$

T_a shall be evaluated based on a 75 year design life.

 1. T_{ult} , Short Term Ultimate Tensile Strength
 T_{ult} is based on the minimum average roll values (MARV)
 2. RF_{cr} , Reduction Factor for Long Term Tension Creep

 RF_{cr} shall be determined from 10,000 hour creep testing performed in accordance with ASTM D5262. A minimum reduction value of 1.60 shall be applied to creep (RF_{cr}).
 3. RF_d , Reduction Factor for Durability
 RF_d shall be determined from polymer specific durability testing covering the range of expected soil environments. $RF_d = 1.10$ minimum.
 4. RF_{id} , Reduction Factor for Installation Damage
 RF_{id} shall be determined from product specific construction damage testing performed in accordance with GRI-GG4. Test results shall be provided for each product to be used with project specific or more severe soil type. $RF_{id} = 1.10$ minimum.
 5. FS , Overall Design Factor of Safety
 FS shall be 1.5 unless otherwise noted for the maximum allowable working stress calculation.
- C. The maximum design tensile load of the geogrid shall not exceed the laboratory tested ultimate strength of the geogrid/facing unit connection divided by a factor of safety of 1.5. The connection strength testing and computation procedures shall be in accordance with NCMA SRWU-1 Test Method for Determining Connection Strength of SRW.
- D. Soil Interaction Coefficient, C_i
 C_i values shall be determined per GRI:GG5 at a maximum 0.75 inch (20 mm) displacement.
- E. Manufacturing Quality Control

The geosynthetic soil reinforcement manufacturer shall have a manufacturing quality control program that includes QC testing by an independent laboratory.

The QC testing shall include:

- Tensile Strength Testing
- Melt Flow Index (HDPE)
- Molecular Weight (Polyester)

2.08 Drainage Pipe

- A. If required, the drainage pipe shall be perforated or slotted PVC pipe manufactured in accordance with ASTM D-3034 or corrugated HDPE pipe manufactured in accordance with ASTM D-1248.

PART 3 EXECUTION

3.01 Excavation

- A. Contractor shall excavate to the lines and grades shown on the construction drawings. Owner's representative shall inspect the excavation and approve prior to placement of leveling material or fill soils. Proof roll foundation area as directed to determine if remedial work is required.
- B. Over-excavation and replacement of unsuitable foundation soils and replacement with approved compacted fill will be compensated as agreed upon with the Owner.

3.02 Base Leveling Pad (if required)

- A. Leveling pad material shall be placed to the lines and grades shown on the construction drawings, to a minimum thickness of 6 inches (15 cm) and extend laterally a minimum of 6 inches (15 cm) in front and behind the modular wall unit.
- B. Soil leveling pad materials shall be compacted to a minimum of 90% Modified Proctor density per ASTM D-1557
- C. Leveling pad shall be prepared to insure full contact to the base surface of the concrete units.

3.03 Modular Unit Installation

- A. First course of units shall be placed on the foundation soils at the appropriate line and grade. Alignment and level shall be checked in all directions and insure that all units are in full contact with the base and properly seated.
- B. Units shall be placed on the foundation soils with a maximum distance of 1/2 inch (1.2 cm) between adjacent units. Vertically adjacent units shall be centered on units above and below. Installation of Candura Stakes is required for the purpose of ensuring proper wall batter and providing "form capacity" and resistance to block sliding during construction. The stakes are inserted through the openings in the bottom rear of the upper block and are driven into the underlying soil. All block layout and placement shall be in accordance with manufacturer's recommendations.
- C. Place and compact fill within and behind wall units. After unit fill is compacted, excess unit fill must be screeded (rod-boarded) off to develop a flat base upon which subsequent

units can be positioned. Place and compact backfill soil behind units. Follow wall erection and unit fill closely with structure backfill.

- D. Maximum stacked vertical height of wall units, prior to unit fill and backfill placement and compaction, shall not exceed one course.

3.04 Geosynthetic Soil Reinforcement Installation

- A. Geosynthetic soil reinforcement shall be oriented with the highest strength axis perpendicular to the wall alignment.
- B. Geosynthetic soil reinforcement shall be placed at the strengths, lengths, and elevations shown on the construction design drawings or as directed by the Engineer.
- C. The Geosynthetic soil reinforcement shall be laid horizontally on compacted backfill and attached to the modular wall units. Place the next course of modular concrete units over the Geosynthetic soil reinforcement. The Geosynthetic soil reinforcement shall be pulled taut, and anchored prior to backfill placement on the Geosynthetic soil reinforcement.
- D. Geosynthetic soil reinforcement shall be continuous throughout the length of embedment. Spliced connections between shorter pieces of Geosynthetic soil reinforcement will not be permitted.

3.05 Reinforced Backfill Placement

- A. Reinforced backfill shall be placed, spread, and compacted in such a manner that minimizes the development of slack in the Geosynthetic soil reinforcement and installation damage.
- B. Reinforced backfill shall be placed and compacted in lift thicknesses not to exceed the "height" of the units being placed. Where heavy compaction equipment is used, compaction lift thicknesses of up to 10 inches (25 cm) may be employed. Lift thickness shall be decreased to achieve the required density as required.
- C. Reinforced backfill shall be compacted to 90% of the maximum density as determined by ASTM D1557. The moisture content of the backfill material prior to and during compaction shall be uniformly distributed throughout each layer and shall be dry of optimum, + 0%, - 3%.
- D. Only lightweight hand-operated equipment shall be allowed within 1 foot (0.3 m) from the back of the modular concrete unit.
- E. Tracked construction equipment shall not be operated directly upon the Geosynthetic soil reinforcement. A minimum fill thickness of 6 inches (15 cm) is required prior to operation of tracked vehicles over the Geosynthetic soil reinforcement. Tracked vehicle turning should be kept to a minimum to prevent tracks from displacing the fill and damaging the Geosynthetic soil reinforcement.
- F. Rubber tired equipment may pass over Geosynthetic soil reinforcement at slow speeds, less than 10 MPH (16 KPH). Sudden braking and sharp turning shall be avoided.
- G. At the end of each day's operation, the Contractor shall slope the last lift of reinforced backfill away from the wall units to direct runoff away from wall face. The Contractor shall not allow surface runoff from adjacent areas to enter the wall construction site.

3.06 Erosion Control

- A. Provide dust and erosion control protection plan in accordance with the contract documents.

3.07 As-built Construction Tolerances

- A. Vertical alignment: ± 1.5 inch (37 mm) over any 10 ft (3 m) distance.
- B. Wall Batter: within 2 degrees of design batter.
- C. Horizontal alignment: ± 1.5 inches (37 mm) over any 10 ft (3 m) distance. Corners, bends, curves: ± 1 ft (0.3 m) to theoretical location.
- D. Maximum horizontal gap between erected units shall be 1/2 inch (1.2 cm).

3.08 Field Quality Control

- A. The Owner shall engage inspection and testing services, including independent laboratories, to provide quality assurance and testing services during construction. This does not relieve the Contractor from securing the necessary construction control testing during construction.
- B. Qualified and experienced technicians and engineers shall perform testing and inspections services.
- C. As a minimum, quality assurance testing should include foundation soil inspection, soil and backfill testing, verification of design parameters, and observation of construction for general compliance with design drawings and specifications.

PART 4 MEASUREMENT AND PAYMENT